

ORANGE COUNTY PUBLIC WORKS

SANTA ANA, CALIFORNIA

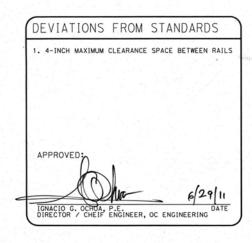
JESS A. CARBAJAL, DIRECTOR

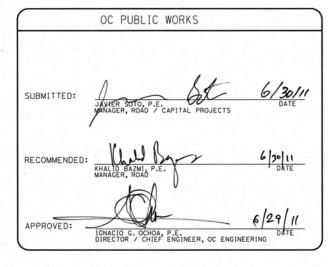
PLANS FOR THE CONSTRUCTION OF

BLACK STAR CANYON ROAD BRIDGES

W.O. # ER03676 SEPTEMBER 2011

FUNDED BY: Orange County Development Agency Fund







INDEX OF SHEETS

SHEET DESCRIPTION

- 1 TITLE SHEET
- 2 GENERAL PLAN BRIDGE BS-5
- 3 GENERAL PLAN BRIDGE BS-4
- 4 GENERAL PLAN BRIDGE BS-3
- 5 DECK CONTOUR
- 6 FOUNDATION PLA
- ABUTMENT DETAILS
- TYPICAL SECTION AND SLAB REINFORCEMENT
- 9 TIE ROD DETAILS
- 10 RAILING WEATHERING STEEL TYPE 115
- 11 RAILING DETAILS
- 12 BAT HABITAT DETAILS
- 13 LOG OF TEST BORINGS BRIDGE BS
- 14 LOG OF TEST BORINGS BRIDGE BS-
- 15 LOG OF TEST BORINGS BRIDGE BS-3

UTILITY OWNER PHONE NO. CONTACT

BASIS OF BEARINGS:

The basis of bearings for this survey is the California Coordinate System, Zone VI, NAD 83, Epoch 1991.35, as determined locally by a line between continuous global positioning stations (CGPS) "CCCS" and MJPK" being N59° 44′52"W as derived from geodetic values published and on file in the office of the Orange County Surveyor.

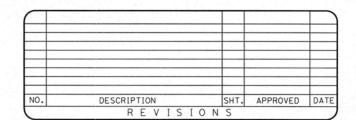
HORIZONTAL DATUM

Coordinates shown are based on the California Coordinate System (CCS83), Zone VI, 1983 North American Datum (1991.35 Epoch). All distances shown are ground, unless otherwise noted Multiply a ground distance by 0.99993790 to obtain a grid distance.

All distances are based on the U.S. Survey Foot.

VERTICAL DATUM

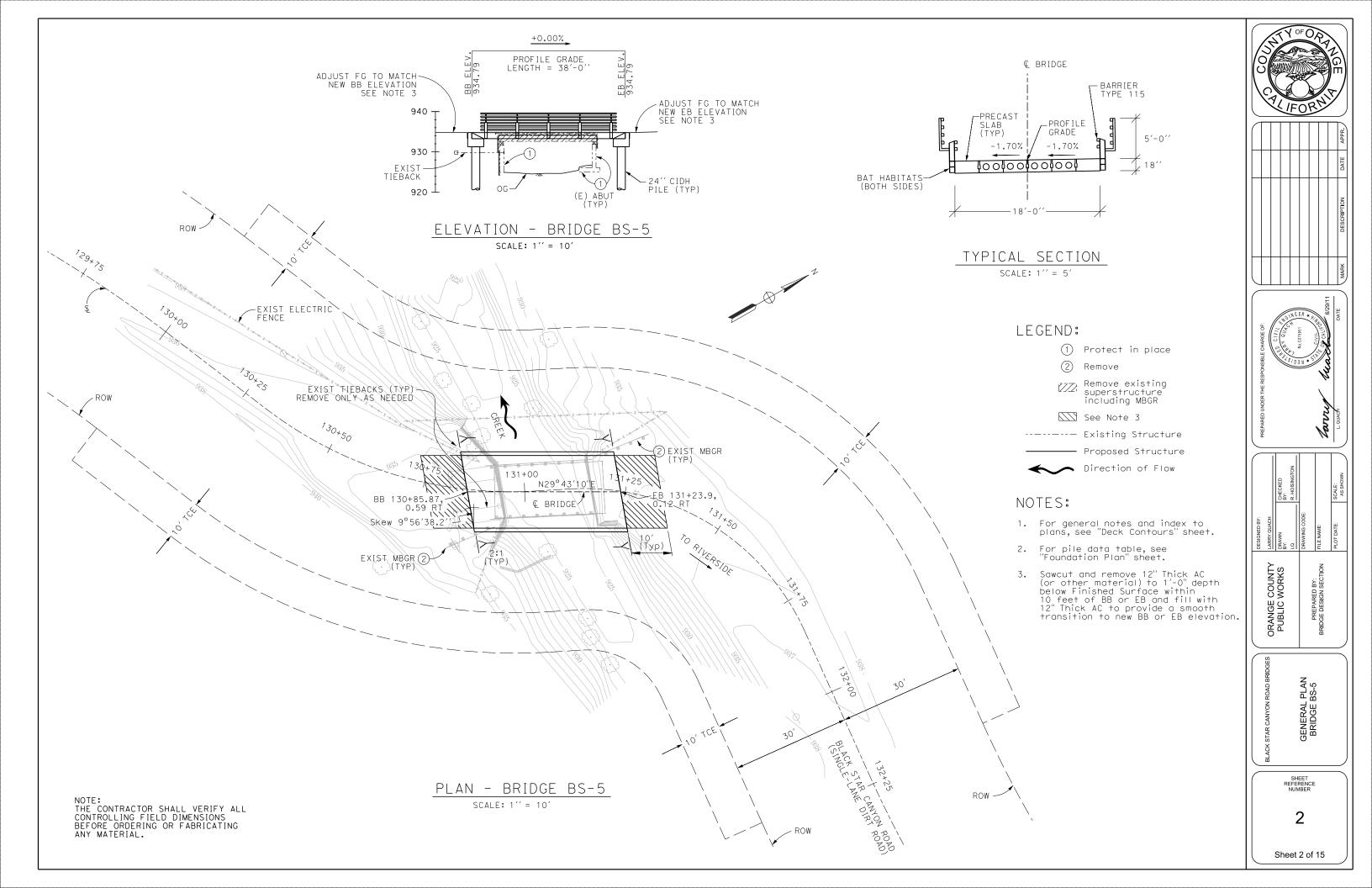
O.C.S. BM. # 3D-108-71 ELEV. = 1029.713 LEVELED 1991
O.C.S. BM. # W-567 ELEV. = 1000.355 LEVELED 1991
NGVD 1929 (1995 O.C.S. ADJUSTMENT)

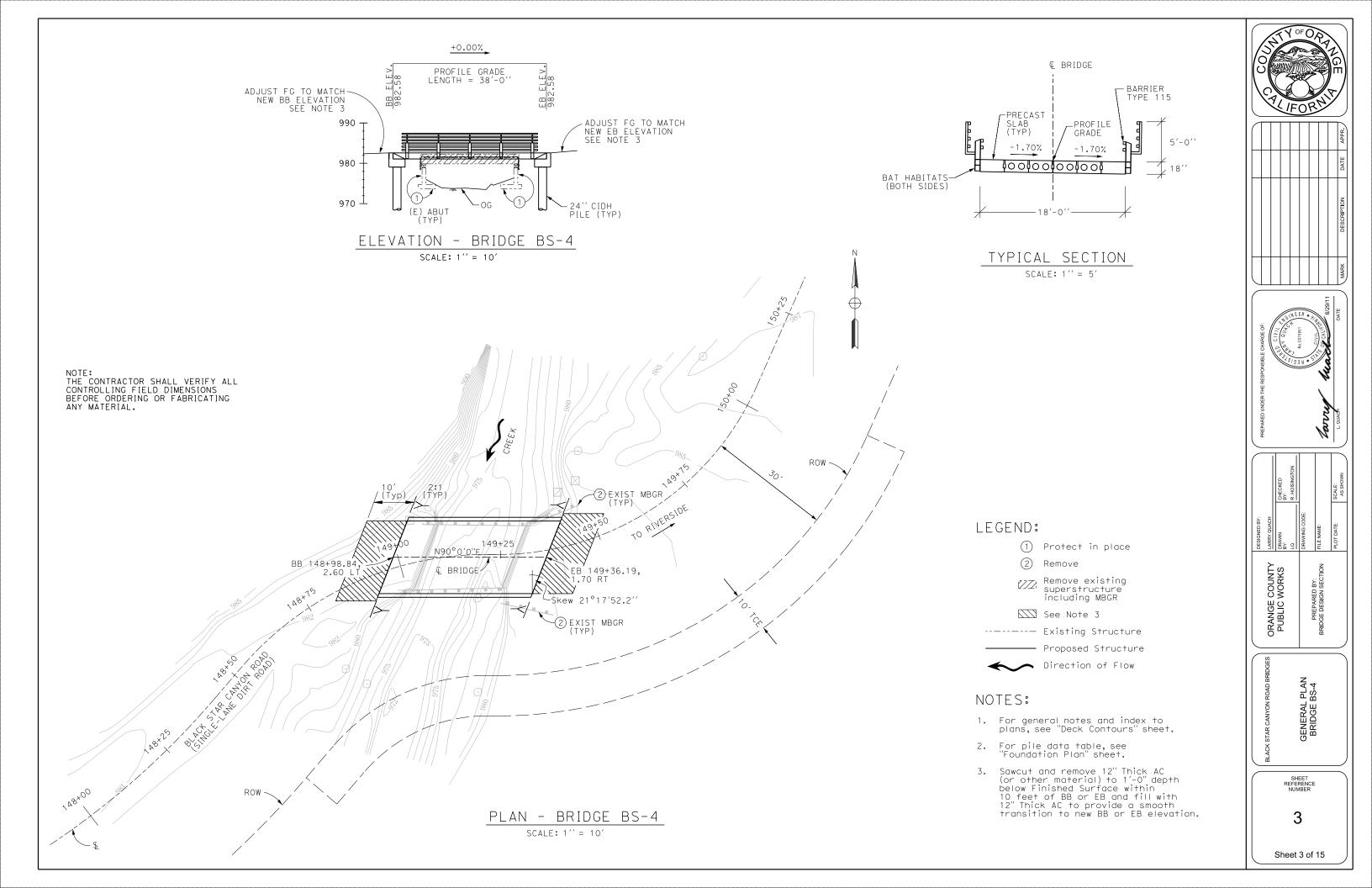


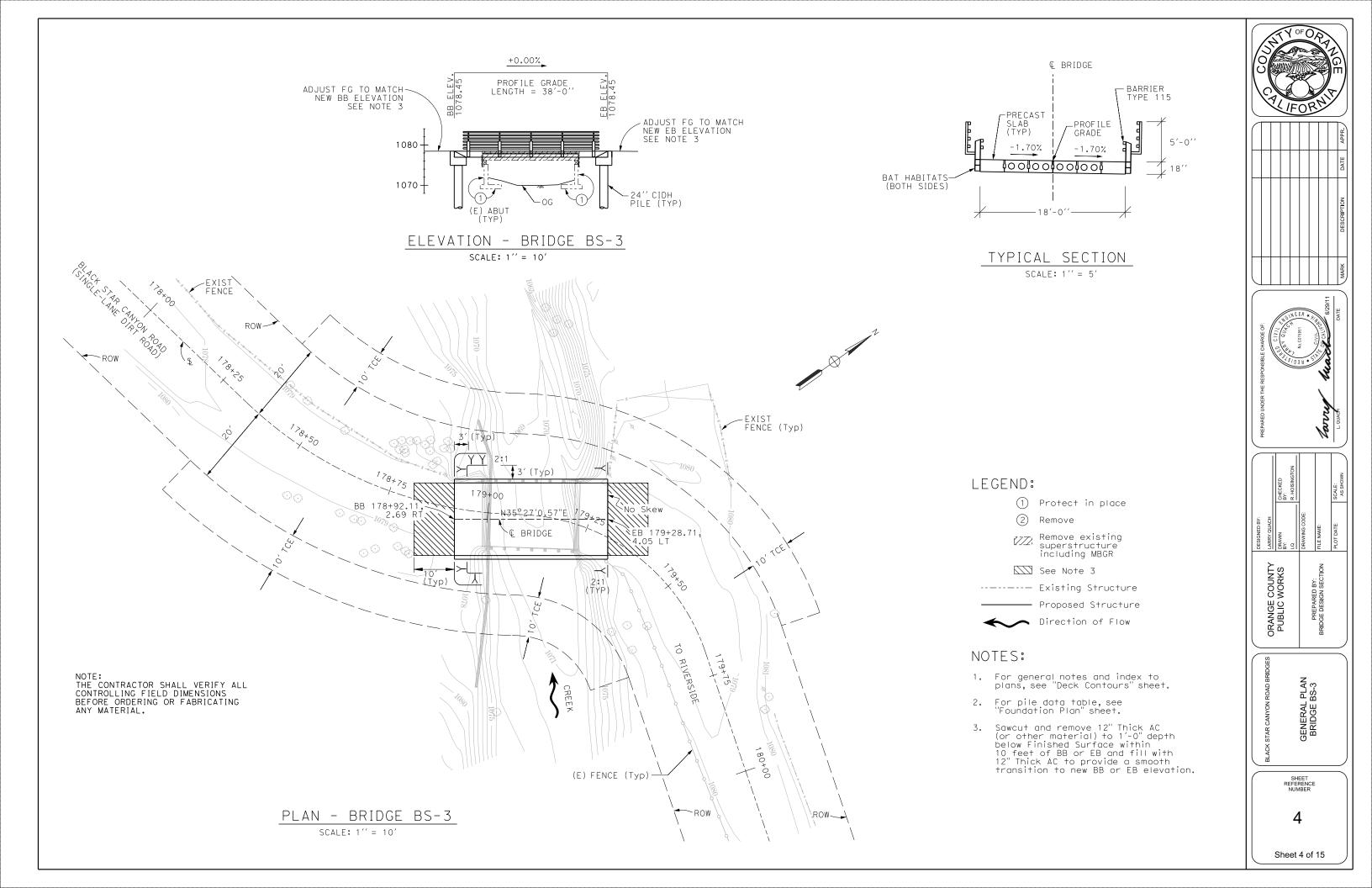
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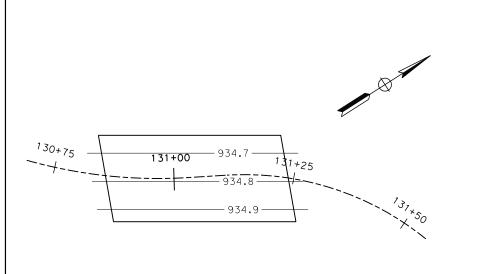
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SHEET 1 OF 15





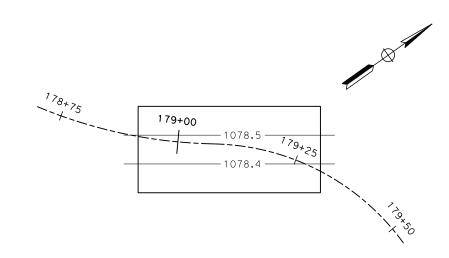




1 A9+50 149+25 982.6-----

BRIDGE BS-4

ABUTMENT (4 ksi @ 28 DAYS)



BRIDGE BS-3

BRIDGE BS-5

PLAN SCALE: 1'' = 10'

NOTES:

PRESTRESSED CONCRETE, BRIDGE (SEE SLAB DETAILS)

CAST-IN-DRILLED-HOLE CONCRETE PILING (4 ksi @ 28 DAYS)

CONCRETE STRENGTH AND TYPE LIMITS

NO SCALE

1. Contour interval is 0.1'
2. Contours do not include camber

GENERAL NOTES LOAD AND RESISTANCE FACTOR DESIGN

DESIGN:

AASHTO LRFD Bridge Design Specifications, 4th edition with Caltrans Amendments; except that geotechnical design of deep foundations, and Standard Bridge Details XS Sheets, are designed using Caltrans Bridge Design Specifications (2000), dated November 2003.

SEISMIC DESIGN:

Caltrans Seismic Design Criteria (SDC), Version 1.6 Dated November 2010.

DEAD LOAD:

Includes 35 psf for future wearing surface.

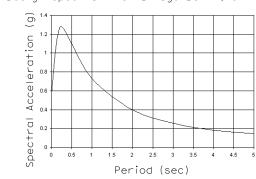
LIVE LOAD:

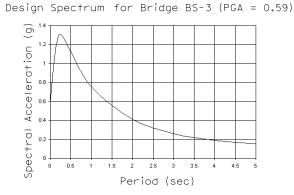
HL93, Permit Design Load, and D9 Bulldozer (as Permit Load).

SEISMIC LOADING:

Soil Profile Type D.

Design Spectrum for Bridge BS-4 (PGA = 0.58)





INDEX TO PLANS

1	TITLE SHEET
2	GENERAL PLAN - BRIDGE BS-5
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5	DECK CONTOURS
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10	RAILING - WEATHERING STEEL TYPE 115
1 1	RAILING DETAILS
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13	LOG OF TEST BORINGS - BRIDGE BS-5
1 4	LOG OF TEST BORINGS - BRIDGE BS-4
15	LOG OF TEST BORINGS - BRIDGE BS-3

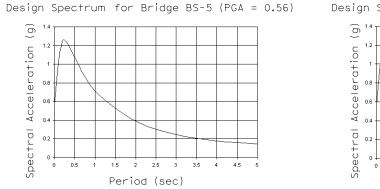
STANDARD PLANS DATED 2006

A10A/B	ABBREVIATIONS
A62C	LIMITS OF PAYMENT FOR EXCAVATION
	AND BACKFILL - BRIDGE
B0-13	BRIDGE DETAILS
B2-3	24" CAST-IN-DRILLED-HOLE CONCRETE PILE
(MOD)	
B6-21	JOINT SEALS (MAXIMUM MOVEMENT RATING = 2")
(RSP)	
B11-51	TUBULAR HAND RAILING

Standard Plan Sheet No. Detail No.

> THE CONTRACTOR SHALL VERIFY ALL CONTROLLING FIELD DIMENSIONS BEFORE ORDERING OR FABRICATING ANY MATERIAL.

0.5 1 1.5 2 2.5 3 3.5 4 4.5

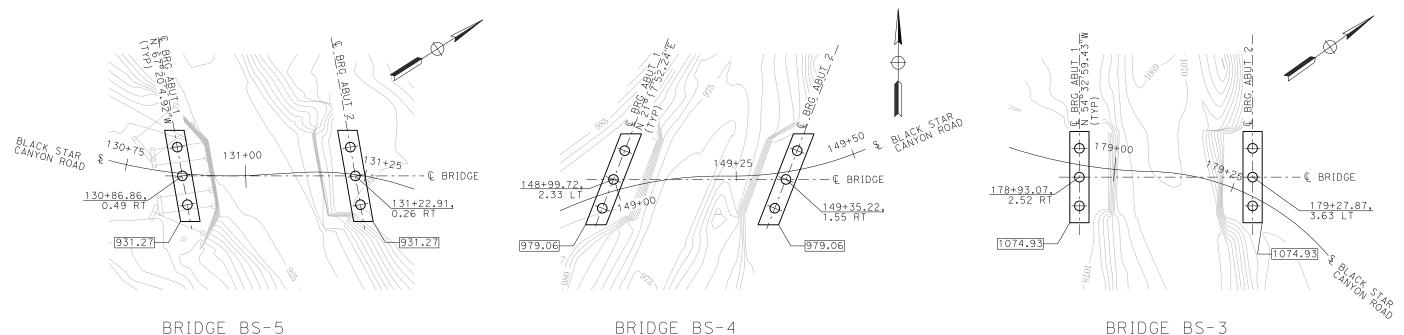


REINFORCED CONCRETE: fy = 60 ksi f'c = See "Concrete Strength and Type Limits"

PRESTRESSED CONCRETE: See Prestressing Notes



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BRIDGE BS-5

PLAN SCALE: 1" = 10'

LEGEND:

INDICATES BOTTOM OF PILE CAP

INDICATES 200 KIP 24"Ø CIDH PILING PER CALTRANS STD PLAN B2-3

Inspection Tubes for 200 Kip 24" CIDH Pile MODIFIED FOR INSPECTION TUBES NTS

HYDROLOGIC SUMMARY HYDROLOGY NOT AVAILABLE

PILE DATA TABLE

8½" Clear Spacing at locations of inspection tubes (typ)

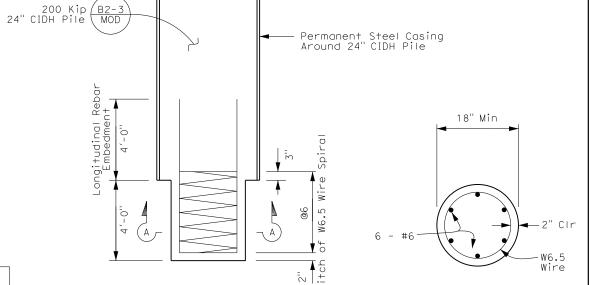
BRIDGE	LOCATION	PILE TYPE	NOMINAL RESISTANCE (KIPS)		DESIGN TIP	SPECIFIED TIP	PERMANENT STEEL CASING	
			COMPRESSION	TENSION ELEVATION (F		ELEVATION (FT)	TIP ELEVATION (FT)	
BS-5	SOUTH ABUT	200 Kip/24" CIDH	180	0	916	916	N/A	
	NORTH ABUT	200 Kip/24" CIDH	180	0	916	916	N/A	
BS-4	WEST ABUT	200 Kip/24" CIDH & Rock Socket	180	0	967	967	971	
	EAST ABUT	200 Kip/24" CIDH	180	0	965	965	N/A	
BS-3	SOUTH ABUT	200 Kip/24" CIDH	310	0	1048	1048	N/A	
	NORTH ABUT	200 Kip/24" CIDH	400	0	1041	1041	N/A	

NOTES:

Inspection Tubes (2" ID) Total 2 <

#8 Total 6

- Design tip elevations are controlled by: (a) Compression, (b) Settlement, (d) Lateral Load.
 The CIDH specified tip elevation shall not be raised.
 No splices in longitudinal rebar allowed.

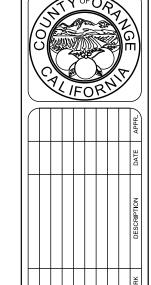


Rock Socket 24" CIDH Rebars Not Shown for Clarity NTS

Section A-A

Bridge BS-4 West Abutment NTS

NOTE:
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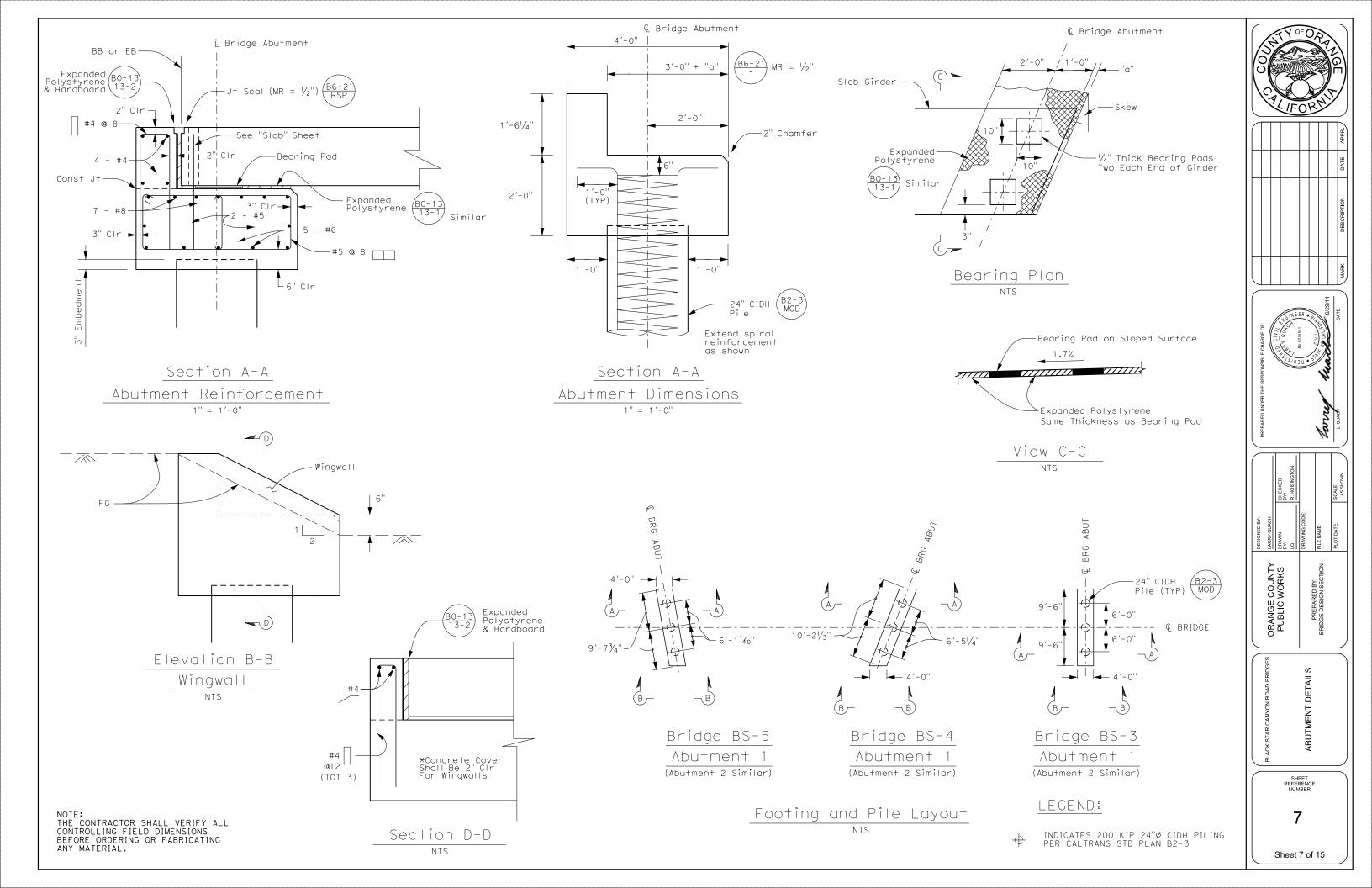
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	LQ	R. HOISINGTON
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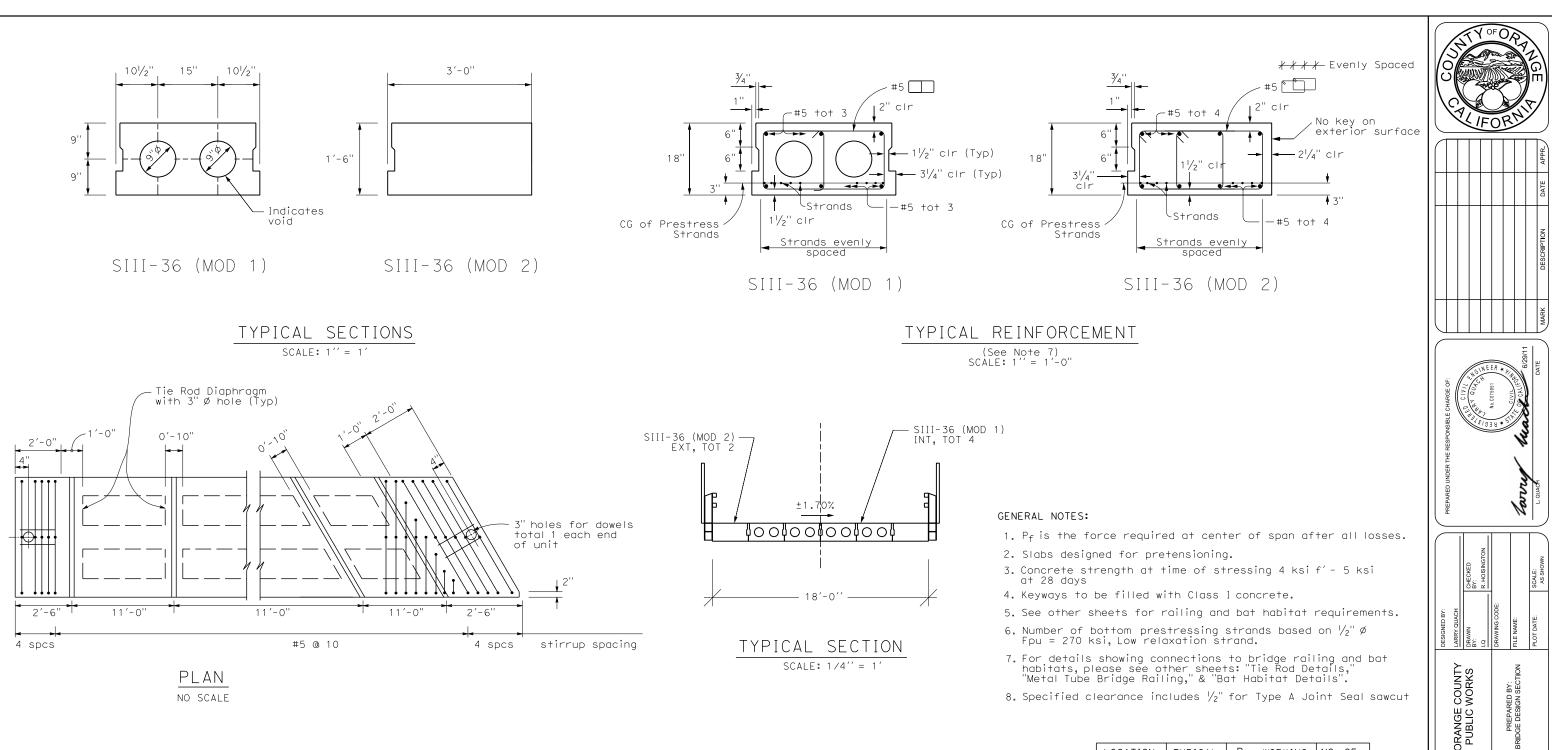
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6

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"L" = Length

ELEVATION NO SCALE

Coarse broom

12"

─1 tot 6 @ each

dowel hole equally spaced (each side of dowel hole)

2" CIr (See Note 8)

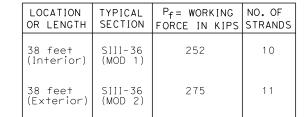
CG of Prestress Strands

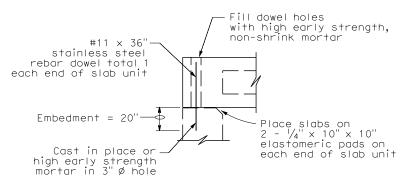
Dowel Hole (Typ)

2" CIr

Note 8)

(See





SUPPORT DETAILS NO SCALE

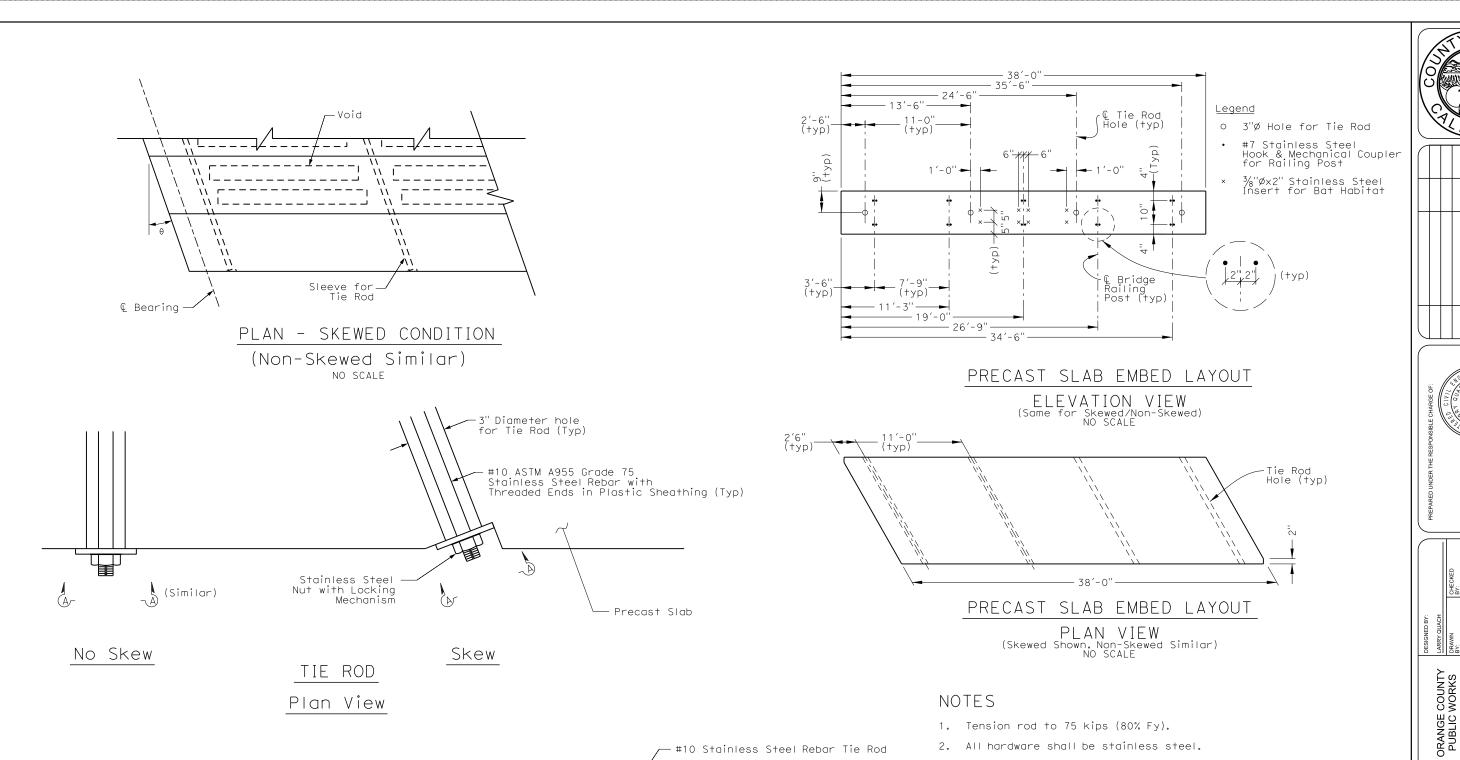
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TYPICAL SECTION AND SLAB REINFORCEMENT SHEET REFERENCE NUMBER

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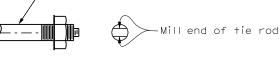
NOTES

- 1. Tension rod to 75 kips (80% Fy).
- 2. All hardware shall be stainless steel.
- 3. Anchorage system (bearing plate, nut, washer and dimensions thereof) is shown for illustration only, to depict the general system. Actual Post-Tensioned Tie Rod Anchorage System shall be designed and detailed by the post-tensioning specialty subcontractor. All hardware shall be stainless steel. Rods shall be 11/4" diameter ASTM A955 Grade 75 (Fy = 75 ksi) stainless steel rebar (#10), total 4 tie rod locations per bridge, as shown on plans. System shall include and specify the following:
 - Adequate thread length at ends of bars Nuts (with locking mechanism) & Washers

 - Nuts (with locking mechanism) & Wasners
 Bearing plate
 Tensioning system and procedure, including any special
 hardware such as direct tensioning indicating washers
 Reinforcement in the voided slabs to conform with
 AASHTO Bridge Design Specifications Article 5.10.9,
 "Post-Tensioned Anchorage Zones"

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TIE ROD END

NOTE:

Prevent tie rod from turning during tensioning by using a wrench on milled end

Stainless Steel Bearing Plate with $1^{1}\!/_{\!2}^{\scriptscriptstyle{\mathrm{T}}}$ \varnothing hole centered for #10 Tie Rod TIE ROD

Elevation A-A

Bearing Surface

for Tie Rod

3"Ø Hole in Precast Slab

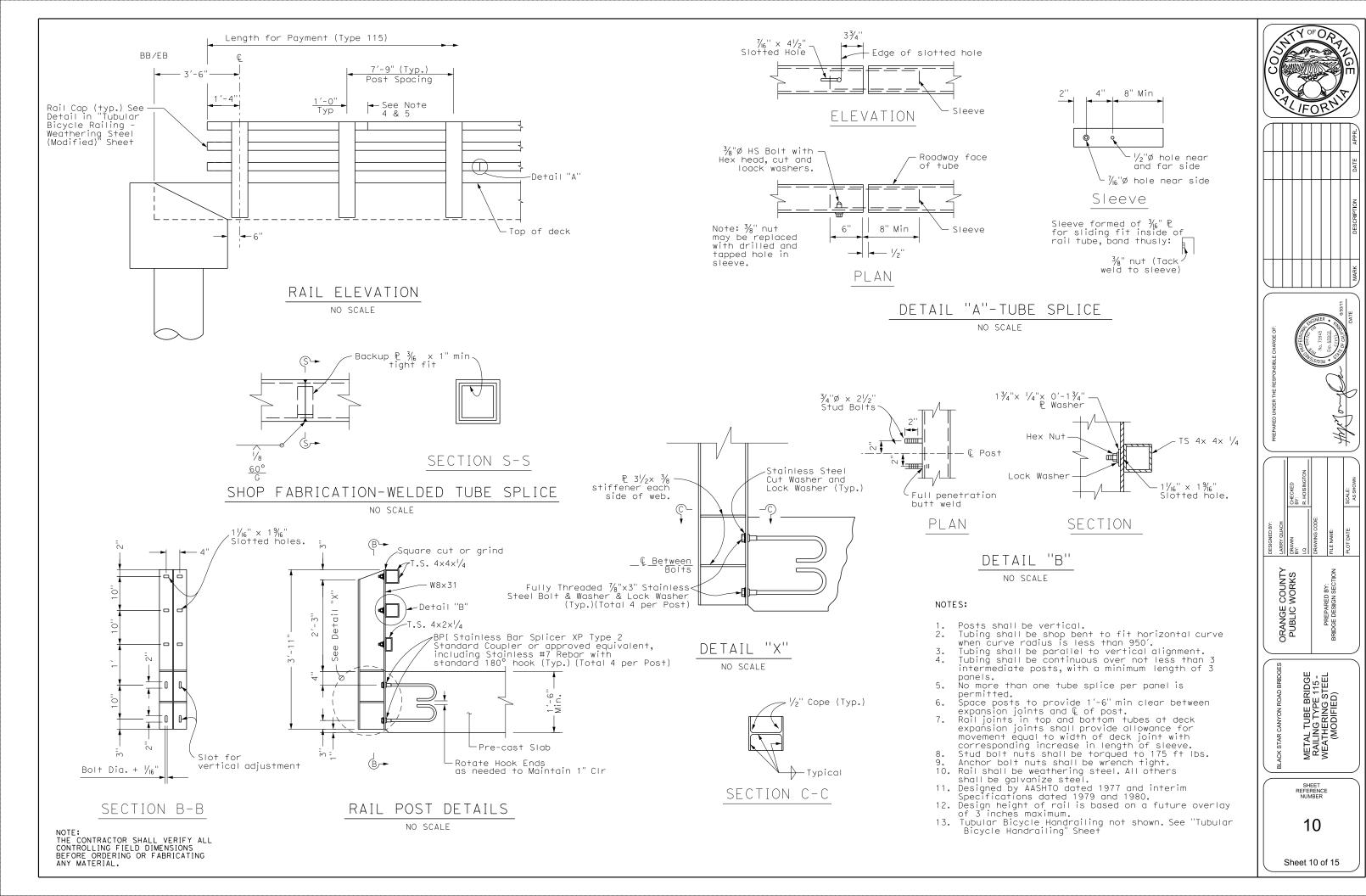
Plan View

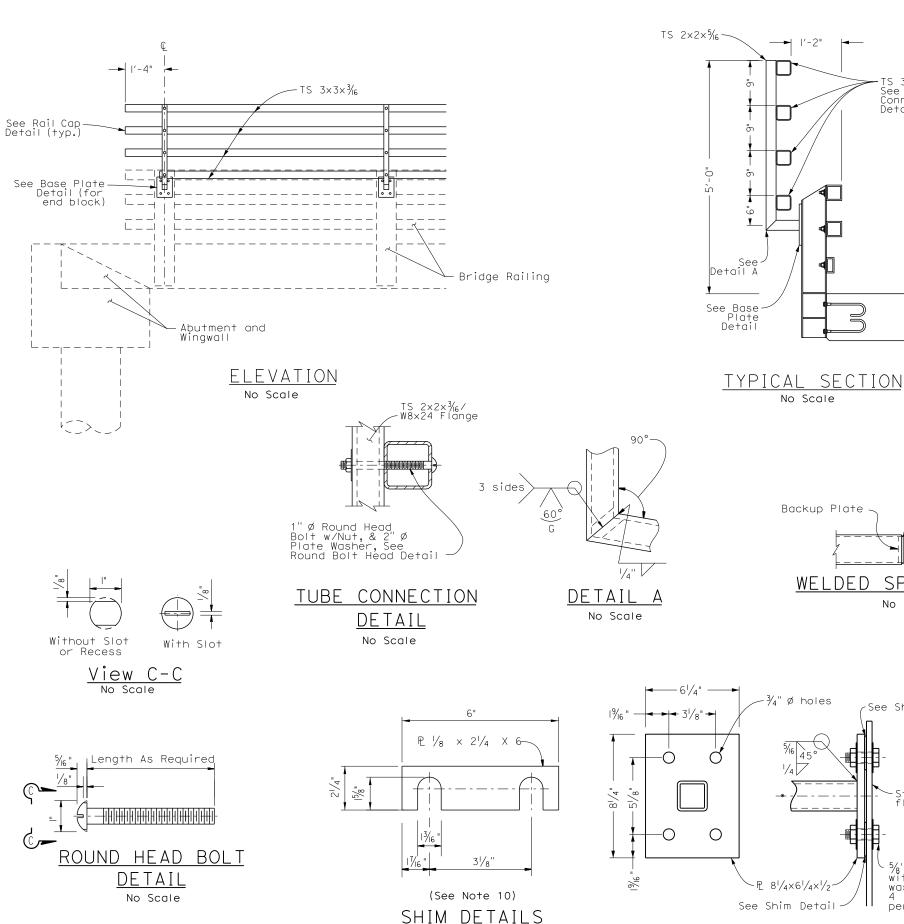


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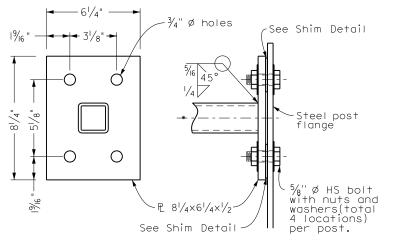




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No Scale



BASE PLATE DETAIL

No Scale

1'-2"

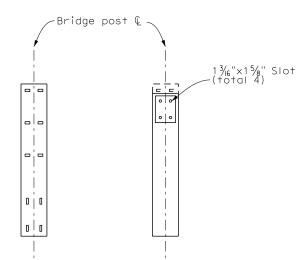
No Scale

Backup Plate

See Tube Connection Detail

WELDED SPLICE DETAIL

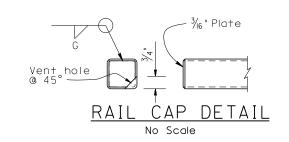
No Scale



(Back view)

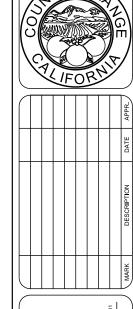
BRIDGE POST ELEVATION No Scale

(Front view)



NOTES:

- 2. Weathering be normal to railing.
- 3. Rail tubes shall be shop bent or fabricated to fit horizontal curve when radius is less than 1000 feet.
- 4. Tube splices shall be located in the tubes spanning deck or wall joints. Increase joint width in tubes to match expansion joint width and increase sleeve length correspondingly.
- 5. Top rail tube shall be continuous over not less than two posts except a short post spacing is permitted near deck or wall joints, electroliers, or other rail discontinuities as noted.
- 6. For details and reinforcement not shown see Standard Plan B11-65.
- 7. See project plans for limits of tubular hand railing.
- 8. Tube rails shall be capped at the ends.
- 9. Details shown are modification/addition to Caltrans Standard Plan B11-65.
- 10. Shim is OPTIONAL, as needed for post plumbness and fitting rail to post/end block alignment.



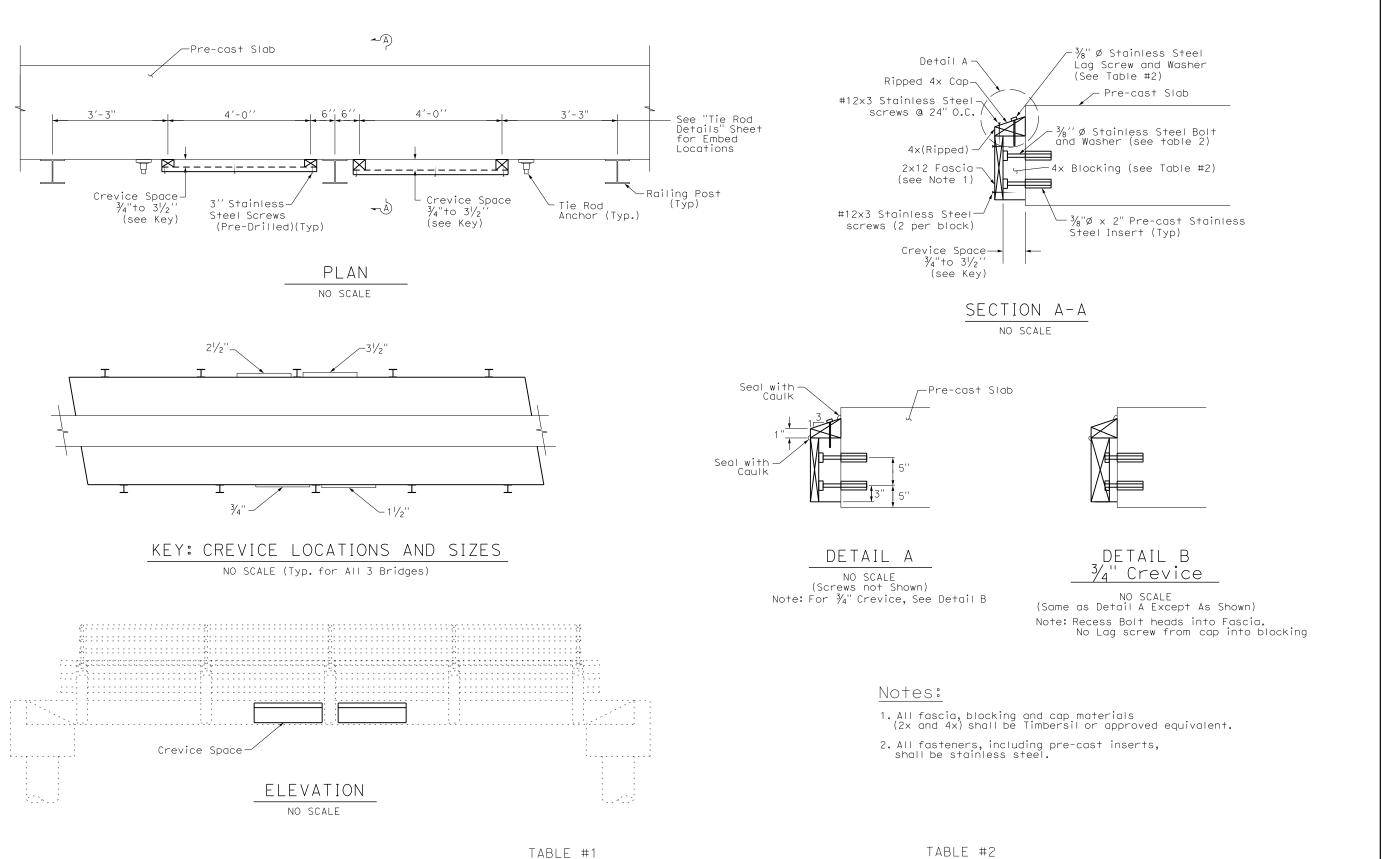


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	Pilot Hole Size - Surface Member	Pilot Hole Size - Substrate Member		
#12 Screws	7/32''	5/32''		
³½" ∅ Lag Screws	3/8''	9/32''		
3%" Ø Bolts	3/8''	N/A		

Crevice Size	3/4''	1 1/2"	21/2''	31/2"
Blocking: Nominal Size	4×1	4×2	4×3	4×4
Bolt Length	3''	3''	4''	5''
Lag Screw Length	N/A	4''	41/2"	5"

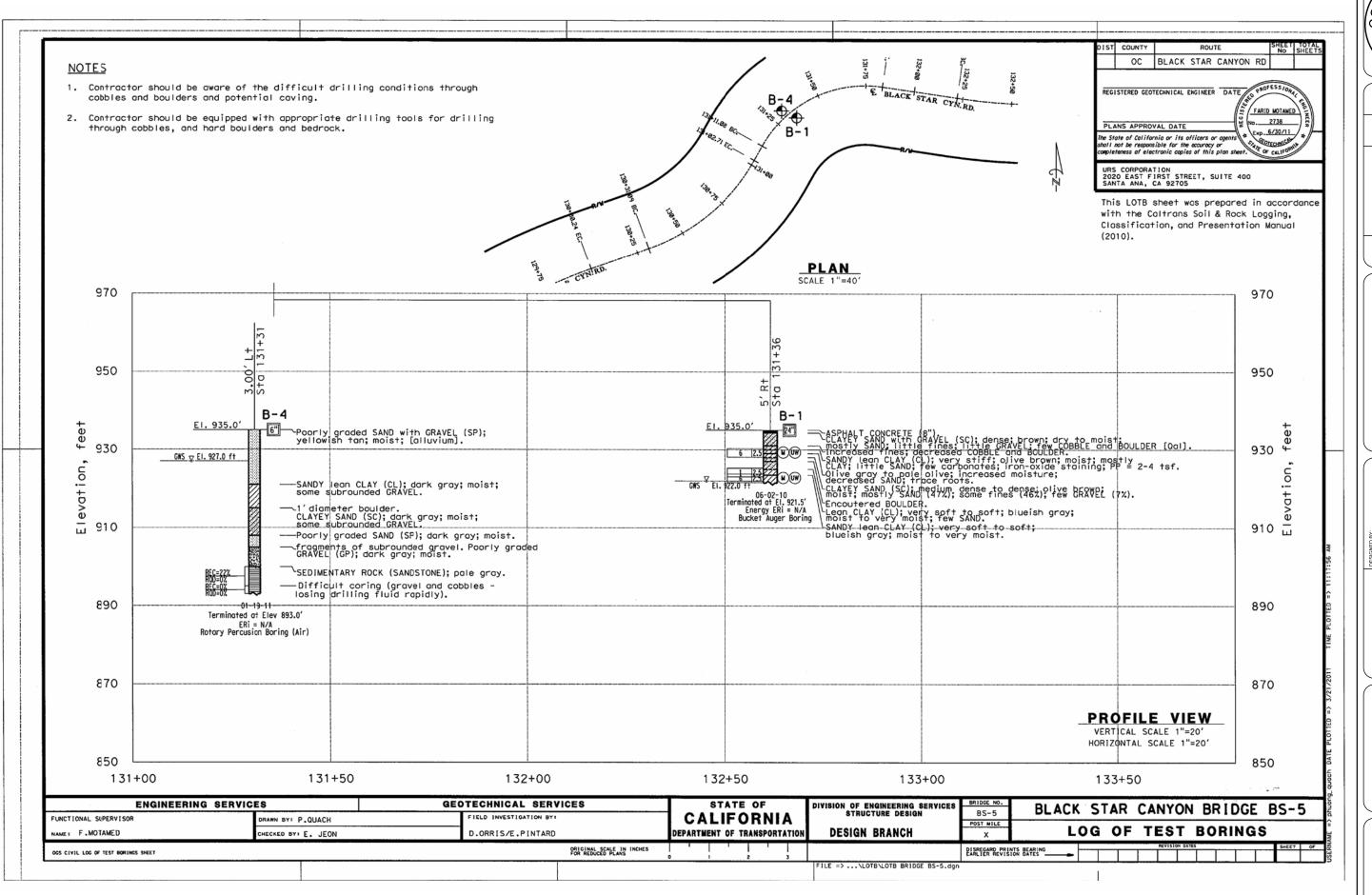
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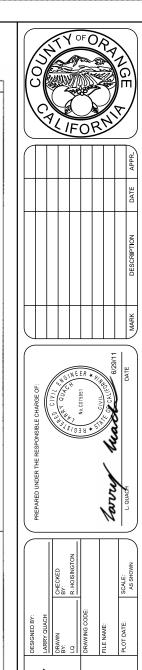
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Sheet 12 of 15

NOTE:
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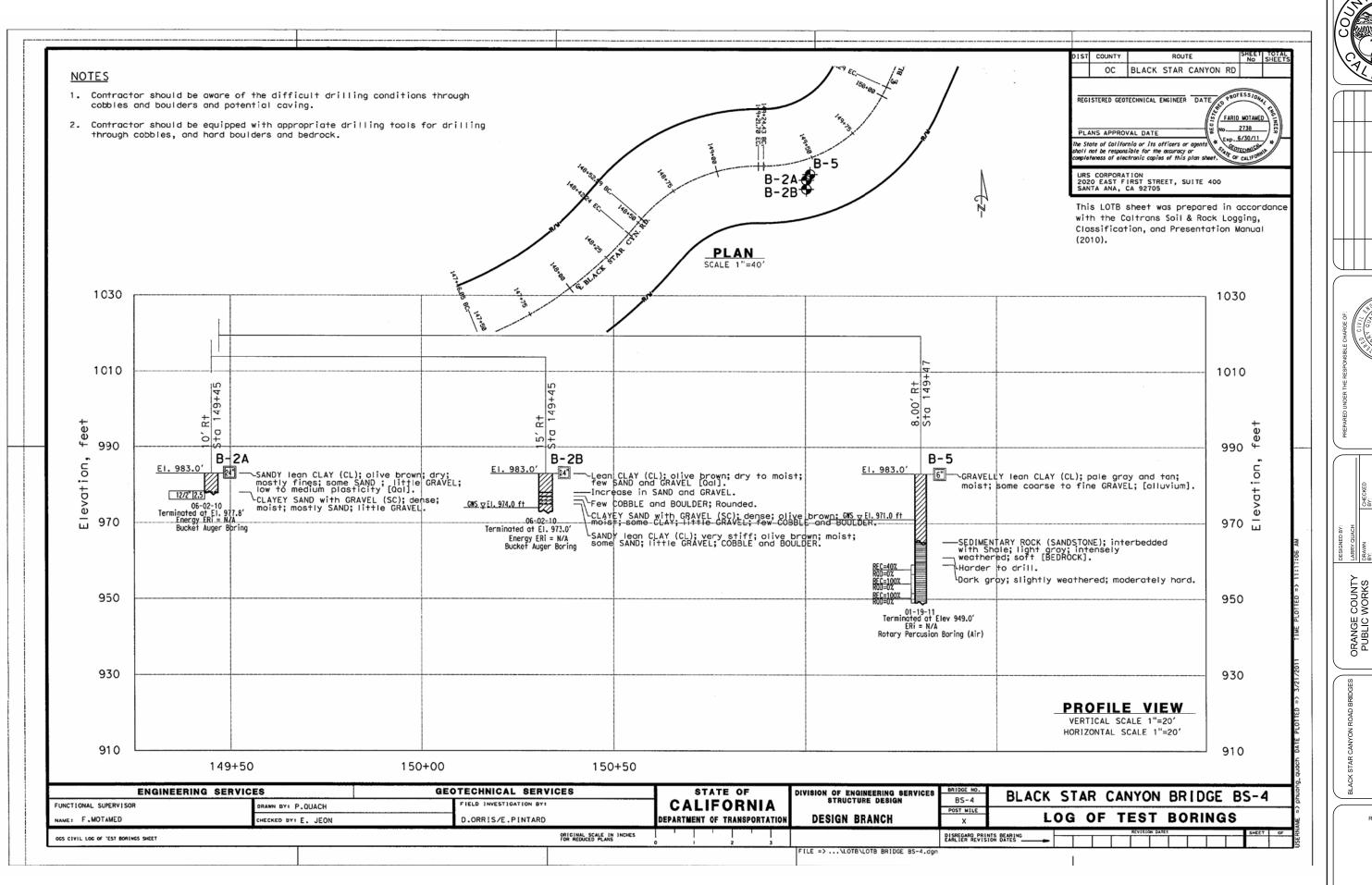


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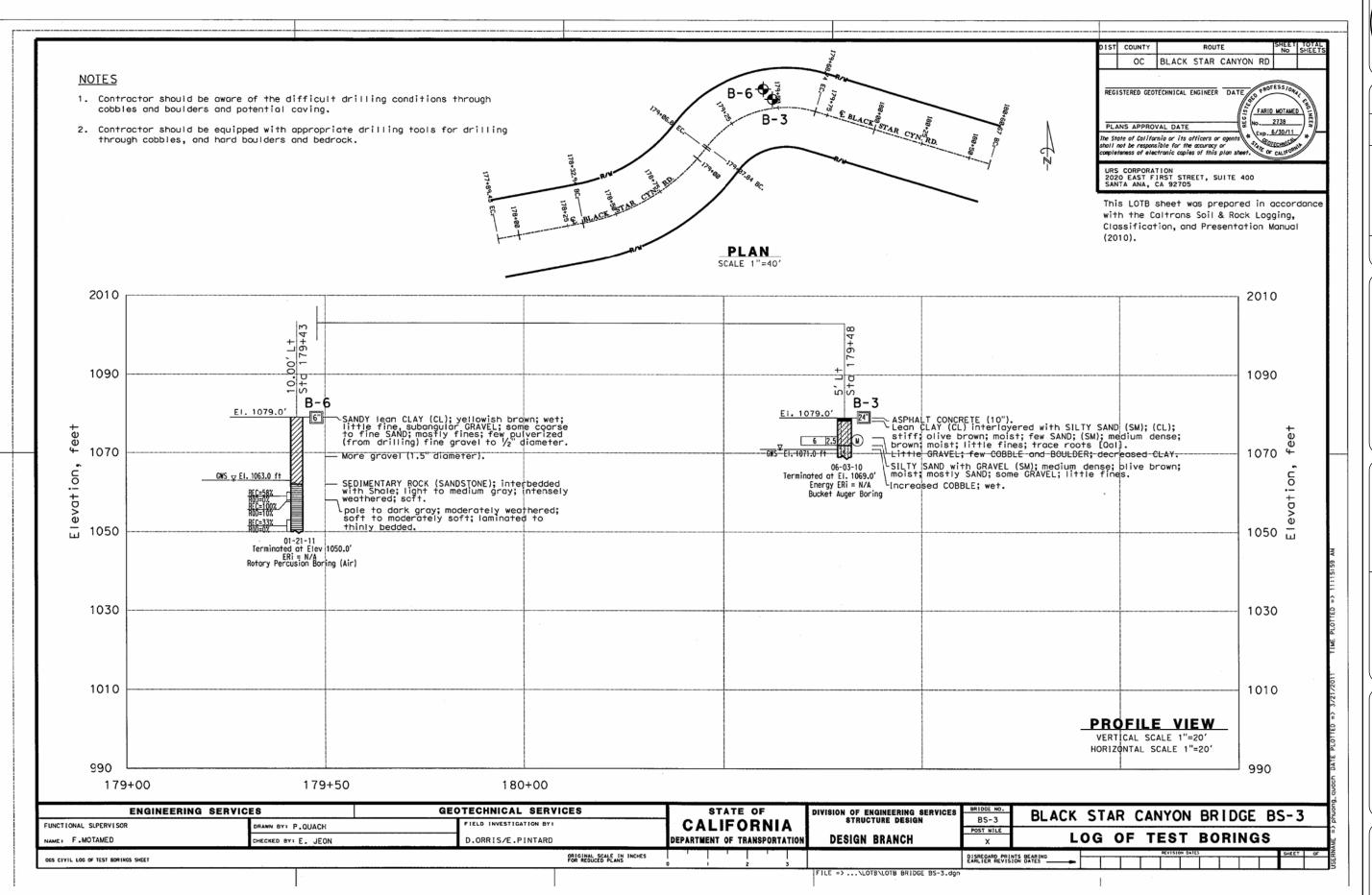


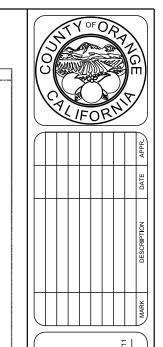
OG OF TEST BORINGS

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